A Comparison between Simulation and Experiment for Hysteretic Phenomena during two-phase Immiscible Displacement

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Abstract

The paper compares a theory for immiscible displacement based on distinguishing percolating and non-percolating fluid parts (Physica A, 371, 209 (2006)) with experimental observations from multistep outflow experiments (Water Resources Res, 27, 2113 (1991)). The paper focusses on hysteretic phenomena resulting from repeated cycling between drainage and imbibition processes in multistep pressure experiments. Taking into account the hydraulic differences between percolating and non-percolating fluid parts provides a physical basis to predict quantitatively the hysteretic phenomena observed in the experiment. \textbf{While standard hysteretic extensions of the traditional theory are nonlocal in time the theory used in this paper is local in time. Instead of storing the pressure and saturation history it requires only the current state of the system to reach the same quantitative agreement.}
1. Introduction

A quantitative prediction of fluid saturation profiles during immiscible displacement remains a fundamental open problem in the physics of porous media. Despite its well known limitations regarding hysteresis and trapping phenomena, the traditional approach [2, 23, 20, 26] has remained the most popular mathematical model for more than 70 years in applications such as reservoir engineering [13] or groundwater hydrology [1].

Many authors have emphasized the importance of hysteresis between drainage and imbibition [25, 7, 21, 18]. Ad hoc extensions of the existing two phase flow model [21, 24, 19, 14] yield reasonable agreement with laboratory measurements. However, the physical foundations for such hysteresis models are not clear.

A recent macroscopic theory of capillarity in porous media [8] proposes to take into account the different hydrodynamic properties of percolating (=connected) and non-percolating (=not connected) fluid parts. This provides the physical basis for hysteresis in our approach. In the residual constitutive relations between relative permeability and saturation as well as between capillary pressure and saturation are recovered including hysteresis [8, 9]. Approximate analytical results for a quasi-static displacement have been calculated in [10]. In [11] and [5], numerical solutions were calculated for experiments with a closed homogeneous column in gravity. Further, analytic and quasi analytic solutions for the theory were developed in [4, 6, 12]. However, an explicit comparison with experimental data is pending.

The objective of this paper is to close this gap. We show that the theory is able to model a laboratory experiment [15] with a porous column that shows hysteretic behavior.

The manuscript is structured as follows. First, we illustrate the experimental setup [15] that has been used to illustrate hysteretic phenomena in fluid distributions in porous media flow. Section 3 briefly presents the theory of percolating and non-percolating phases and its mathematical formulation for incompressible immiscible two-phase flow. In Section 4 we discuss the numerical representation of the experiment and its implementation. The results are presented and discussed in Section 5.

2. Experimental Setup

The experimental setup of [15] consists of a vertical cylindrical, 72 cm long column, whose side walls are impermeable, filled with a homogeneous, isotropic and incompressible porous medium, an unconsolidated sandy material comprising approximately 97.5, 0.8 and 1.7% sand-, silt-, and clay-sized particles respectively. The two fluids considered here are water and air and water is the wetting fluid. The top of the column is connected to the atmosphere so that only air can enter the top of the column. The bottom of the column is connected to a water tank and only water can enter from the bottom. The pressure in the tank is adjustable. Initially, the porous column is completely water saturated and the pressure \( P_W \) in the water tank is chosen such that it compensates the water column \( P_W(t = 0) = 72 \text{ cmH}_2\text{O}(= 7.06 \text{ kPa}) \).

Hence, the capillary fringe is located at the top of the column. The pressures in this section are given in cm column of water because this translates one to one to the position of the water table in the column. The pressures in the upper and lower reservoir vs. time.

Figure 1. Conceptual picture of the experimental setup and the pressure protocols in the upper and lower reservoir vs. time.

The column is drained by lowering the pressure in the lower reservoir by \( \Delta P = 5 \text{ cmH}_2\text{O} \) every \( \Delta t = 10 \text{ min} \) 13 times until the water pressure reaches \( P_W = 7 \text{ cmH}_2\text{O} \). After a relaxation period of \( \Delta t = 50 \text{ min} \), the water pressure is raised again in seven steps by \( \Delta P = 5 \text{ cmH}_2\text{O} \) every \( \Delta t = 10 \text{ min} \) and the column is imibed again. After \( \Delta t = 50 \text{ min} \), the water pressure is again lowered by \( \Delta P = 5 \text{ cmH}_2\text{O} \) every \( \Delta t = 10 \text{ min} \) for 5 times and the column is drained again. Finally, \( \Delta t = 50 \text{ min} \) later, the water pressure is raised again by \( \Delta P = 5 \text{ cmH}_2\text{O} \) steps every \( \Delta t = 10 \text{ min} \) until it reaches its initial value \( P_W(t > 8 \text{ h}) = 72 \text{ cmH}_2\text{O} \).

However, the original water content in the column is not...
recovered because a fraction of air remains trapped in the medium [7].

[681.1.7.1] In the laboratory, water saturation and water pressure have been measured at height \( x = 0.4 \) m, \( x = 0.5 \) m, \( x = 0.6 \) m and \( x = 0.7 \) m from the bottom of the column. [681.1.7.2] It may safely be assumed that the air pressure is essentially hydrostatic and atmospheric for two reasons: Firstly, because of the high viscosity and density contrast (\( \mu_w = 0.001 \) kg m\(^{-1}\) sec\(^{-1}\), \( \mu_0 = 18 \times 10^{-6} \) kg m\(^{-1}\) sec\(^{-1}\) and \( \varrho_w = 1000 \) kg m\(^{-3}\), \( \varrho_0 = 1.2 \) kg m\(^{-3}\)), and secondly, because the column is short. [681.1.7.3] It is therefore concluded that the measurement of water saturation and water pressure suffices to determine the capillary pressure saturation relationship.

3. Definition of the Model

[681.1.8.1] In this section, a brief summary of the mathematical model is given. [681.1.8.2] The following equations are based on volume, mass and momentum balance equations analogous to the foundations of the traditional theory (see e.g. [9] for a succinct but detailed parallel development). [681.1.8.3] It is assumed that both fluids are incompressible and immiscible and that the lateral dimensions of the column are small with respect to the capillary fringe. [681.1.8.4] Hence, a one dimensional description is an appropriate approximation. [681.1.8.5] The mass balances for the four fluid phases (percolating water is identified by the index 1, non-percolating water by 2, percolating oil by 3 and non-percolating oil by 4) read as

\[
\begin{align*}
\varrho_w \frac{\partial q_1}{\partial t} + \varrho_w \frac{\partial S_1}{\partial x} &= M_1, \\
\varrho_w \frac{\partial q_2}{\partial t} + \varrho_w \frac{\partial S_2}{\partial x} &= M_2 = -M_1, \\
\varrho_0 \frac{\partial q_3}{\partial t} + \varrho_0 \frac{\partial S_3}{\partial x} &= M_3, \\
\varrho_0 \frac{\partial q_4}{\partial t} + \varrho_0 \frac{\partial S_4}{\partial x} &= M_4 = -M_3, 
\end{align*}
\]

where \( t \) denotes time, \( x \) denotes position or height along the column, \( \phi \) denotes porosity, \( \varrho_w, \varrho_0 \) denote the density of water, respectively air, \( S_i \) the saturation, \( q_i \) the volume flux and \( M_i \) the mass exchange term of the phase \( i \). [681.1.8.6] The mass exchange term accounts for the fact that percolating and non-percolating phases of the same fluid exchange mass by break-up and coalescence. [681.1.8.7] The mass exchange terms take the form

\[
M_1 = \eta_3 \phi \varrho_w \left( \frac{S_1 - S_2^*}{S_w^* - S_w} \right) \frac{\partial S_w}{\partial t},
\]

\[
M_3 = \eta_4 \phi \varrho_0 \left( \frac{S_3 - S_3^*}{S_w^* - S_w} \right) \frac{\partial S_w}{\partial t},
\]

with the parameter functions

\[
S_w^* = (1 - \min (S_{0, \text{im}}, (1 - \epsilon_M S_0))) \Theta (\partial_t S_w) + \min (S_{w, \text{dr}}, (1 - \epsilon_M S_w)) [1 - \Theta (\partial_t S_w)],
\]

\[
S_2^* = \min (S_{w, \text{dr}}, (1 - \epsilon_M S_w)) [1 - \Theta (\partial_t S_w)],
\]

\[
S_3^* = \min (S_{0, \text{im}}, (1 - \epsilon_M S_0)) [1 - \Theta (\partial_t S_0)],
\]

where the parameter \( S_{0, \text{im}} \) is a limiting saturation for the non-percolating air, \( S_{w, \text{dr}} \) a limiting saturation for non-percolating water and \( \Theta (\cdot) \) denotes the Heaviside step function. [681.1.8.8] Water saturation is given by \( S_w = S_1 + S_2 \) and the air saturation by \( S_0 = S_3 + S_4 \). [681.1.8.9] The parameter \( \epsilon_M \approx 0 \) is a mathematical regularization parameter. [681.1.8.10] It allows to simulate also primary processes. [681.1.8.11] The volume fluxes of the phases take the form

\[
\begin{pmatrix}
q_1 \\
q_2 \\
q_3 \\
q_4
\end{pmatrix}
= \Lambda
\begin{pmatrix}
-\partial_x P_1 - \varrho_w g \\
-\partial_x (P_3 - \gamma P_2^2 S_2^{-1}) - \varrho_w g + P_a \partial_x S_1^\alpha \\
-\partial_x P_3 - \varrho_0 g \\
-\partial_x (P_1 - \delta P_4^2 S_4^{-1}) - \varrho_0 g + P_b \partial_x S_3^\beta
\end{pmatrix}
\]

(4)

where \( P_1, P_3 \) denote the averaged pressures of the percolating phases, \( g \) the gravity acceleration, \( \alpha, \beta, \gamma, \delta \), \( P_a, P_b, P_2^*, P_4^* \), are constitutive parameters. [681.1.8.12] The parameters \( \alpha, \beta, P_a, P_b \) are associated with capillary potentials and \( \gamma, \delta, P_2^*, P_4^* \) with the energy stored in the interface between the non-percolating phases and the surrounding percolating phases of the other fluid [9]. [681.1.8.13] A generalized mobility matrix is denoted by \( \Lambda \) with the coefficients

\[
\Lambda_{ij} = \phi^2 S_i S_j [\tilde{R}^{-1}]_{ij},
\]

where \( [\tilde{R}^{-1}]_{ij} \) denote the components of the inverse of the viscous coupling parameter matrix. [681.1.8.14] A comparison with the classical two-phase Darcy equations yields that \( \tilde{R}_{11} \approx \mu_w/k \) and \( \tilde{R}_{33} \approx \mu_0/k \), where \( k \) denotes the permeability of the porous medium. [681.1.8.15] The system of equations is closed with the volume conservation for incompressible fluids and incompressible porous media

\[
S_1 + S_2 + S_3 + S_4 = 1
\]

plus a special form of the general self-consistent closure condition [11, 5].
The list of parameters with units and their numerical values used for simulating the experiment. Note that \( \epsilon_M \) is a mathematical regularization parameter, i.e. the limit \( \epsilon_M \rightarrow 0 \) is implicit and it has been tested that the numerical results do not change in this limit.

<table>
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<tr>
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<td>( \circ )</td>
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<td>( \alpha )</td>
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<td>( P_a )</td>
<td>( P_b )</td>
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<tr>
<td>( \gamma )</td>
<td>( \delta )</td>
<td></td>
</tr>
<tr>
<td>( P^*_a )</td>
<td>( P^*_b )</td>
<td>Pa</td>
</tr>
<tr>
<td>( R_{11} )</td>
<td>( R_{33} )</td>
<td>kg m(^{-3}) sec(^{-1})</td>
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<td>( R_{22} )</td>
<td>( R_{44} )</td>
<td>kg m(^{-3}) sec(^{-1})</td>
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<tr>
<td>( R_{ij} ), ( i \neq j )</td>
<td></td>
<td>kg m(^{-3}) sec(^{-1})</td>
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</table>

The resulting capillary pressure curves are compared in Figure 2. [681.1.10.4] The initial conditions are \( S_1(x,0) = 0.997, \ S_2(x,0) = 0.001, \ S_3(x,0) = 0.001, \ S_4(x,0) = 0.001 \) for all \( x \in [0 \text{ cm}, 72 \text{ cm}] \). [681.1.9.14] Initial conditions for the pressures are not required because of the implicit formulation. [681.1.9.16] Before the protocol for the pressure is started, the system is given one day under hydrostatic water pressure conditions to equilibrate.

\[
P_3 = P_1 + \frac{1}{2} \left( P_a S_1^{\alpha} - P_b S_3^{\beta} + \gamma P^*_2 S_2^{\gamma - 1} - \delta P^*_4 S_4^{\delta - 1} \right)
\]

for the pressures of the percolating phases.

4. Simulation Setup

In this section we discuss how the experiment is represented mathematically. [681.1.9.3] The four mass balance equations (1) are solved numerically. [681.1.9.4] First, equations (6) and (7) are used to eliminate \( S_4 \) and \( P_3 \). [681.1.9.4] The primary variables are \( S_1, S_2, S_3 \) and \( P_1 \). [681.1.9.5] The mass balances are discretized in space by cell centered finite volumes with upwind fluxes. [681.1.9.6] They are discretized in time with a first order implicit fully coupled scheme. [681.1.9.7] The corresponding system of nonlinear equations is solved with the Newton-Raphson method. [681.1.9.8] The whole scheme is implemented in MATLAB. [681.1.9.9] The simulation is run with a resolution of one cell per centimeter, i.e. with \( N = 72 \) collocation points. [681.1.9.10] Details of the algorithm are given elsewhere [3].

[681.1.9.11] Dirichlet boundary conditions for the pressure \( P_1 \) of the percolating water phase are imposed at the lower boundary \( (x = 0 \text{ m}) \), where pressure is determined by the water reservoir. [681.1.9.12] Dirichlet boundary conditions for the atmospheric pressure \( P_3 \) of the percolating air phase are chosen at the upper boundary \( (x = 0.72 \text{ m}) \) of the column. [681.1.9.13] All the other boundaries are impermeable so that the flux across them must vanish.

[681.1.10.1] The parameters for the simulation are given in Table 1. [681.1.10.2] They were obtained by fitting the primary drainage curve of the capillary pressure saturation relationship obtained in the residual decoupling approximation [9] to the primary drainage curve of van Genuchten parameterization that [15] obtained by a fit to data of the first drainage process in the experiment. [681.1.10.3] The van Genuchten parameters in [15] are \( \alpha^{dr} = 4.28 \times 10^{-4} \text{ Pa}^{-1}, \ \alpha^{im} = 8.56 \times 10^{-4} \text{ Pa}^{-1}, \ n^{dr} = 5.52, \ n^{im} = 5.52, \ m^{dr} = 0.82, \ m^{im} = 0.82, \ S_{wi} = 0.17, \ S_{sr} = 0.25 \). [681.1.10.4] The resulting capillary pressure curves are compared in Figure 2. [681.1.10.5] The viscous resistance coefficients were obtained through \( R_{11} \approx \mu_{w}/k, \ R_{33} \approx \mu_{O}/k \), where \( k = 33.7 \times 10^{-12} \text{ m}^2 \) was again taken from [15]. [681.1.10.6] The viscous resistance coefficients for the non-percolating phases are assumed to be much larger than those for the percolating phases \( R_{22}, R_{44} \gg R_{11}, R_{33} \). [681.1.10.7] For the time-scale of the experiment the results do not depend on the numerical values of the resistance coefficients given in Table 1 [3].
5. Results

The results of the simulations are compared with experimental data and the most sophisticated model of [15] (previously developed in [22, 17]) in Fig. 3 and 4. Figure 3 shows the computed time evolution of $S_{W}(x, t)$, $S_{2}(x, t)$, $S_{4}(x, t)$ at $x = 0.4\,\text{m}$, $x = 0.5\,\text{m}$, $x = 0.6\,\text{m}$ and $x = 0.7\,\text{m}$ as continuous and dashed lines. The experimental data for $S_{W}$ are shown as plus signs and circles and the simulation results with the most sophisticated model of [15] are represented by dash-dotted and dotted curves. There is good qualitative agreement at all four positions.

During the first two hours of the experiment, the saturation decreases because of the lowered pressure in the water reservoir. The instants at which the capillary fringe passes the measurement points agree at all four points between simulation and measurement. The decrease in water saturation induces a production of non-percolating water. The corresponding decrease in non-percolating air is not visible in the graph because of the small amount of initial non-percolating air. Hence, the rate of saturation change decreases and saturation reaches an almost stationary value. The calculated water saturations of that plateau are slightly smaller than the experimental ones. Measured and predicted values during the first increase of the reservoir pressure at around $t \approx 4\,\text{h}$ show good agreement excluding $x = 0.70\,\text{m}$. In contrast to the simulated saturation, the measured saturation decreases at $x = 0.70\,\text{m}$ after an increase of the pressure at the lower boundary. This surprising effect was not discussed in the experimental work [15] and the lack of error bars makes an interpretation difficult (similar experiments [16] by the same authors suggest error bars of order $\Delta S_{W} \approx 0.1$). The increase of water saturation induces a decrease of non-percolating water and a production non-percolating air.

The subsequent evolution of the saturation shows qualitatively similar phenomena upon changing the pressure. Note, that the spatiotemporal evolution of the saturation requires an infinite number of scanning curves in traditional hysteresis modelling, i.e. when the process changes between drainage and imbibition as a function of time and position.

Here in our model a single set of parameters is sufficient, and there is no need to know scanning curves beforehand. At the end of the experiment water is almost completely connected at all four measurement positions, while most of the air is disconnected and trapped. The trapped air prevents the complete filling of the column with water and indicates the irreversibility of the process.

Figure 4 illustrate the hysteretic drainage and imbibition processes in the $P_{c} - S_{W}$ plane. It shows $P_{c}(S_{W})$ obtained by eliminating $x$ and $t$ from the measurements of $-P_{c}(x, t)$ and $S_{W}(x, t)$ at $x = 0.4\,\text{m}$, $x = 0.5\,\text{m}$, $x = 0.6\,\text{m}$ and $x = 0.7\,\text{m}$. The lines of the simulations indicate the time evolution which is compared to the time discrete observations in the experiment. Experimental and simulated data agree qualitatively. The values at $S_{W} \approx 0.99$ corresponds to the minimum in the pressure protocol of the water reservoir (see Figure 1) at the start of the experiment. The decrease in water saturation induces an increase in $P_{c}$ during the primary drainage and the curves of all four positions coincide with the experimental data. The point with the
lowest saturation of each primary drainage branch corresponds to the instant when the pressure in the water reservoir is increased again. \[681.1.14.8\] The deviations between simulation and experiment at \(x = 0.7\text{ m}\) and \(x = 0.5\text{ m}\) originate from the mismatched saturations (see Fig. 3). \[681.1.14.9\] The following imbibition discloses the hysteretic nature of the process as the curves at the four different positions follow different scanning curves. \[681.1.14.10\] Also the subsequent drainage yields different drainage scanning curves. \[681.1.14.11\] This branch of the curve is difficult to identify in the experimental data at \(x = 0.7\text{ m}\) and \(x = 0.6\text{ m}\) but matches well at the other two locations. The final imbibition yields saturation \(S_W \approx 0.8\) and the curves at all four positions almost coincide again.

\[681.1.15.1\] The figures show that the quality of our results is comparable to the most sophisticated model used in [15]. \[681.1.15.2\] However, in our theory the hysteresis in saturation profiles is due to the natural assumption that breakup and coalescence rates are proportional to the rate and direction of saturation change, whereas in their models, the hysteresis is nonlocal in time and inserted directly by hand into the constitutive functions. \[681.1.15.3\] The good quality of the results is surprising because it is evident that some of our assumptions such as the incompressibility of air and the incompressible porous medium are questionable. \[681.1.15.4\] We remark also, that the lack of error bars for the experimental data makes an interpretation difficult.

6. Conclusions

\[681.1.16.1\] In summary, we presented a comparison of experimental observations with theoretical predictions for a theory of two-phase flow in porous media based on the hydraulic differences between percolating and non-percolating fluid parts. \[681.1.16.2\] It was shown that taking into account these differences renders the modeling of a multistep outflow capillary pressure saturation relation measurement possible with one single set of parameters. \[681.1.16.3\] In contrast to most of the hysteretic extension of the standard theory for two-phase flow in porous media, the underlying theory has a physical foundation. \[681.1.16.4\] We therefore conclude that percolation of fluid phases may be the physical reason for hysteresis between drainage and imbibition during immiscible displacement.

Acknowledgments

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Appendix

\[681.1.16.5\] This appendix considers some computational aspects of our theory to aid readers simulating experiments with hysteresis. \[681.1.16.6\] It clarifies fundamental differences (locality vs. non-locality) between the theory presented here and a traditional hysteresis model. \[681.1.16.7\] The natural assumption in eq. (2), that breakup or coalescence of ganglia is proportional to the rate and direction of saturation changes, is neither equivalent nor related to the traditional hysteretic extensions of capillary pressure or relative permeabilities.

\[681.1.17.1\] The origin of hysteresis in the present theory (see [9, 10]) differs fundamentally from traditional hysteresis models such as the model utilized in [15]. \[681.1.17.2\] Traditional hysteresis models require to store for each location inside the
sample the pressure and saturation history (i.e. the reversal points, where the process switches between drainage and imbibition). [681.1.17.3] In our theory such pressure and saturation histories are not needed. [681.1.17.4] Instead, contrary to traditional hysteresis models, our theory allows to compute the future state of the porous medium, given only the knowledge of its present state. [681.1.17.5] In other words: While traditional hysteresis models are nonlocal in time (and thus require to memorize the systems history), our theory is local in time.

[681.1.18.1] In practical computations the locality of our theory translates into reduced storage requirements and a more straightforward implementation. [681.1.18.2] Table 2 below lists the fields (i.e. the position and time dependent quantities) necessary to compute the future time evolution of the system.

[681.1.19.1] Both approaches need a pressure and saturation field at the present time instant \( t \). [681.1.19.2] Our theory needs in addition the unknowns \( S_2, S_4 \) to completely specify the present state of the system. [681.1.19.3] This amounts to two additional state variables at each collocation point. [681.1.19.4] Traditional hysteresis models need in addition 2\((x_i)\) historic values, one for pressure and one for saturation, at each collocation point \( x_i \). [681.1.19.5] The number \( l(x_i) \) is the number of time instants \( t_j(x_i), j = 1, ..., l(x_i) \) at which reversals occur at position \( x_i, i = 1, ..., N \). [681.1.19.6] A reversal is a switching between drainage and imbibition at the collocation point \( x_i \). [681.1.19.7] The number \( l(x_i) \) depends on the nesting or not of scanning curves. [681.1.19.8] The number \( l(x_i) \) and the time instants \( t_j(x_i) \) are not known in advance. [681.1.19.9] In [15] it is assumed \textit{ad hoc} that nested loops do not occur and that the last two reversals are sufficient to avoid pumping effects. [681.1.19.10] This uncontrolled approximation might fail for experiments with cyclic pressure changes where nested scanning loops are expected to occur locally. [681.1.19.11] In the general case [15] expect that the last four or five reversals are sufficient.

[681.1.20.1] Finally, it may be of interest for practical computations that the model of [15] postulates explicitly and implicitly numerous functional relations between the variables and unknowns characterizing the state of the system. [681.1.20.2] Examples are not only the capillary pressure-saturation relationship or the relative permeability-saturation relation, but also the functional relations between the various effective, apparent, entrapped and historic saturations and pressures \( \overline{S}_w, \overline{S}_w, h_{nw}, \overline{S}_{nt} \), \( \overline{S}_{nt}, \Delta h_{nw} \) and \( \overline{S}_{w} \) appearing in [15]. [681.1.20.3] The functional forms for these relationships are postulated purely theoretically and have, apparently, been tested by inverse fitting but not yet by a direct experimental test. [681.1.20.4] The large number of such functional relations and the freedom to parametrize them results in so many possible fit parameters for the model of [15] that a meaningful comparison to other approaches based on the number of free fit parameters becomes difficult.

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<tr>
<th>this theory</th>
<th>[15]</th>
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<tr>
<td>( S_w(x_i, t) )</td>
<td>( S_w(x_i, t) )</td>
</tr>
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<td>( S_2(x_i, t) )</td>
<td>—</td>
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<tr>
<td>( S_4(x_i, t) )</td>
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<tr>
<td>( P_3(x_i, t) )</td>
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<td>( \Delta \overline{S}_w l(x_i) (x_i, t(x_i)(x_i)) )</td>
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</tbody>
</table>

Table 2. List of unknowns needed at a given time instant \( t \) to compute the future time evolution for the mathematical model in this paper as compared to the mathematical model of [15]. Quantities corresponding to each other appear in the same row. The arguments \( x_i \) denote \( N \) discretized positions, i.e. collocation points \( i = 1, ..., N \) of the numerical simulation. The notation of in the right column follows [15], the notation in the left column is that of this paper. The time instants \( t_j(x_i), j = 1, ..., l(x_i) \) are the time instant of the \( j \)-th reversal at position \( x_i \). The number \( l(x_i) \) of reversals (nested scanning curves) depends on position \( x_i \).

References


